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(88,195) Wt.51,085-18, 45,000, 12/18. A AR W.

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ALL DOMMUNICATIONS
TO BE ADDRESSED TO THE
CROWN AGENTS FOR THE COLORISMS,
THE ABOVE REFERENDE AND THE
DATE OF THIS LETTER BEING QUOTED.

TELEGRAMS, "OROWN, LONDON," TELEPHONE 2780 VIOTORIA (6 LIBER) State 3

34268 Rec^o IN SEF 14

WHITEHALL GARDENS,

LONDON, 8, W.

9th September 1914.

Sir,

In reply to your letter No. 20651 of 16th
June 1914 I have the honour to enclose for the information of the Secretary of State a copy of preliminary observations by Messrs. Rendel Palmer and
Tritton as to the most suitable type of locomotive for
use on the Uganda Railway, the possibility of the
prompt supply of such locomotives, and the ability
of the bridges to stand the load.

I have the hondur to be,

Sir.

Your obedient Servant,

for Crown Agents.

de united Bounetary of state,

Colonial Office.

/283/7

MESSRS RENDEL PALMER & TRITTON TO CROWN AGENTS

Dartmouth Street,
Westminster, S.W.
5th September 1914.

24268 Rec^o 10 SEP 1

UGANDA RAILWAY STRENGTH OF BRIDGES

Your letter No. 283/E/7 of 23rd June 1914

Gentlemen

We have now made an examination of all the different types of bridges on the line, and also of the trestles of the Kikuyu and Mau Viaducts, with a view to ascertaining their camability for carrying a heavier live load than now in force. The load we have assumed consists of two 4-8-2 type locomotives, as shewn in Mr. Neville's sketch which accompanied your letter, each weighing 58t tons (with 10 tons on each of the four driving axles) and having a tender weighing 32 tons, or 90t tons in all for each engine and tender, followed by a train load averaging 1t tons per foot run, but subject to the proviso that no wagon axle load shall exceed 10 tons, nor be closer to an adjacent 10 ton axle than 4' 0".

2. Under this loading we find that with some few exceptions tize whole of the structures are within the limits laid down for steel bridges in this country. The points of excess are:-

- (1, In some of the trestles of the Kikuyu Viaducte where <u>some</u> of the longitudinal diagonal ties might be stressed up to 6% tons per square inch by the traction or breaking effort of a passing train, instead of to 6% tons permissible, and
- (2) In various plate girders and in the cross and rail girders for the 100 ft. spans, where the bearing pressure on the rivets connecting the main angle bars to the web plates would exceed 6½ tons; per square inch.
- 3. In no case, however, do we consider that the stresses indicated are so high as to call for any reinforcement. We think therefore that the proposed 4-8-2 locomotives (with four driving axles of 10 tons each) may be sanctioned for use over the whole line, subject to a speed restriction of 15 miles per hour over the 100 ft. spans (of which we believe there are only one or two cases on the line) and 25 miles per hour over the viaducts.
- 4. We are sending this portion of our report in, advance because we understand that it is desirable to settle the question of the engines referred to at once. The question of the heavier (15 tons) axle loads is a greater one, and will be investigated and reported on as soon as possible.
- 5. We believe from recent offers we have received for locomotives that engines of the type proposed by the Locomotive Superintendent could be obtained with delivery commencing inabout eight months, though of course under present conditions it is very difficult to give a reliable estimate in this respect.

U G A N D A R A I L W A Y

24268

Summary of Maximum Stresses produced in Bridges by two 4-8-2 type locomotives each weighing 90½ tons including tender, and train of wagons at 1½ tons per foot (with no Impact) and wind at 56 lbs. per square foot.

WAKUPA (OR "SALISBURY")BRIDGE

Main Girders (60 ft. span)	tons per
Tension	5.38
Compression	4.62
Shear	2.24
Bearing (Rivets)	7.22

Cross Girders

Tension		4.21
Compression		3.70
Shear		1.77
Bearing (Rivets)	•	5.12

Rail Girders

Tension	or	Compression	4.83
Shear			1.51

KIKUYU VIADUCTS

40 ft. spans

Tension

5.

		Ţ	ons per
	Compiess	ion	5.32
	Shear		2.0
	Bearing	(Rivets)	7.86
O ft. Spa	ns.		
	Tension		3.1
	Compress	ion	2.73
	Shear		1.19
	Bearing	(Rivets)	5.39
restles			ų ·
	Leg. Com	pression	5.4
	Bracing	(transverse)	
		Tension	5.96
		Compression	1.80
	Bracing	(longitudinal)	
		Tension	6.75
		Compression	0.6
8			1
O ft. Sp	ans		
	Tension		4.95
	Compress	sion	4.68
	Shear ,		2.03
	Bearing	(Rivets)	7.0
20 ft. 8r	ens.		
7	Tension	And the second second second	3,15
1	Compres	Mon	2.73
<	Shear		1.19
- 'a Kin	Bearing	Rivetal	5.18

MAU VIADUCT

···		sq. in.
Cross Gir	ders (Channel Bars)	
	Tension or Compression	5.8
Trestles		
12	Leg, Compression	5.82
	Bracing (transverse)	
	Tension	5.37
	Compression	1.70
	Bracing (longitudinal)	
	Tension	5.75
	Compression	0.6
		Tone per sq.in.
6 It. CI	ear Span (4rg. No. 92)	
	Tension or Compression	2.79
	Shear	1.88
10 ft. C	lear Span (drg. No. 209)	ē.
	Tension or Compression	3.89
	Shear	¥4 1
12 ft. C	lear Span (drg. No. 93)	Sa.
1	Tension or Compression	4.14
,	Shear	1.30

	Tons per sq. in.
20 ft. Clear Span (Drg. No. 94)	
Tension of Compression	4.13
Shear	1.65
40 ft, Clear Span (drg.No. 95)	
40 rt, Clear Span (drg. No. 90)	
Tension	5.05
Compression	4.98
Shear	1.74
Bearing (Rivets)	6.0
40 ft. Clear Span (drg. No. 743)	
Tension	4.85
Compression	4.91
Shear	1.65
Bearing (Rivets)	6.13
50 Pt. Cleur Span (drg. No. 251)	
Tensi en	5.12
A Compression	4.9
Shear	1.77
Bearing (Rivets)	7.8
Bearing Girders for 40 ft. Spans [drg. No. 235]	
Tension or Compression	4.28
Shear	2.7.
Bearing Girders for 60 ft. spans [drg. No. 616]	
Tension or Compressian	4.46
Shear.	3.43

100 ft. Clear Spans (drgs. Nos. 595 & 596)

Tons per sq. in.

Main Girders

Tension	6.5
Compression	5.61
Bearing (Rivets)	4.87

Cross Girders

Tension	5.53
Compression	4.52
Shear	2,62
Bearing (Rivets)	9.9

Rail Girders

Tension	3.31
Compression	2.91
Shear	1.93
Bearing (Rivets)	7.3